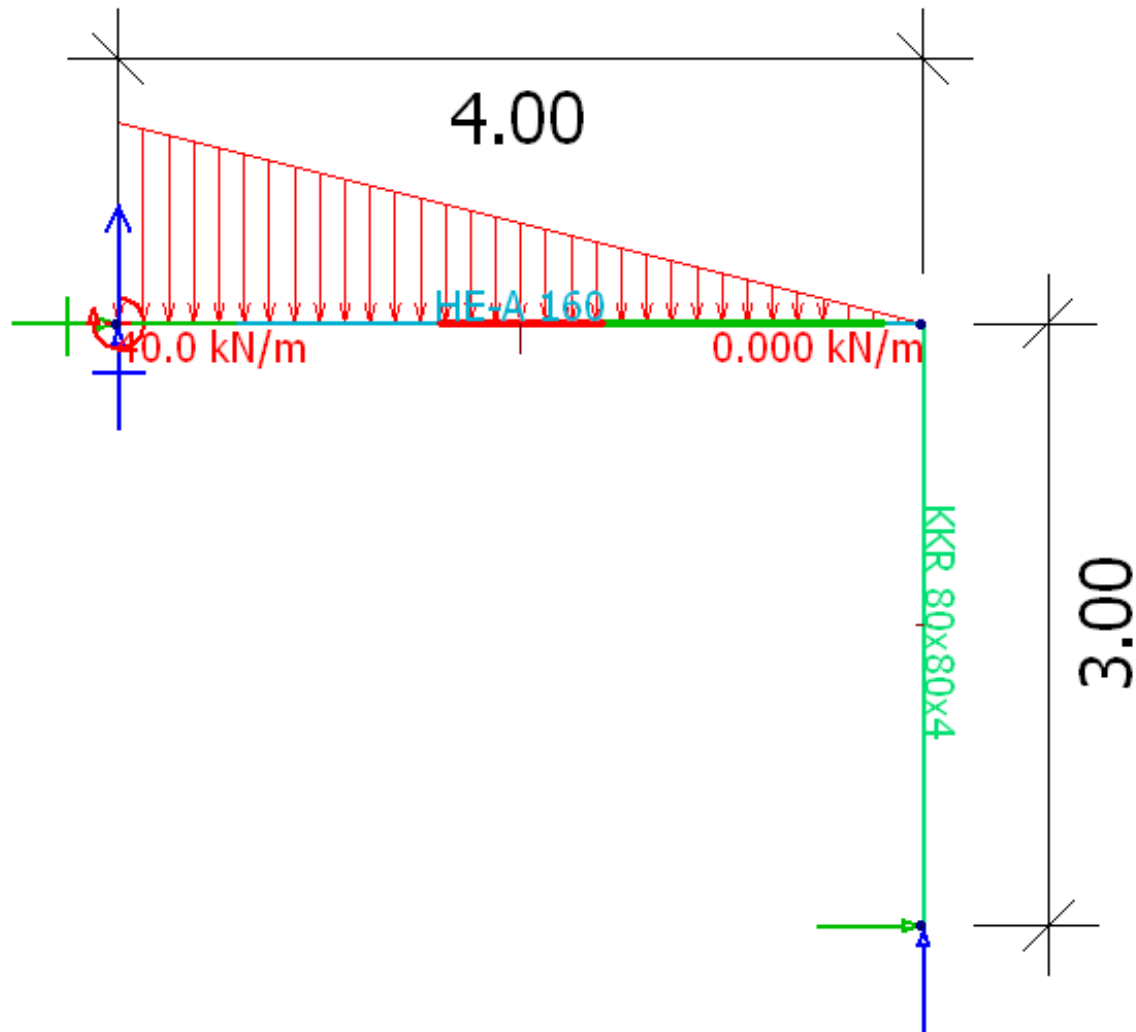
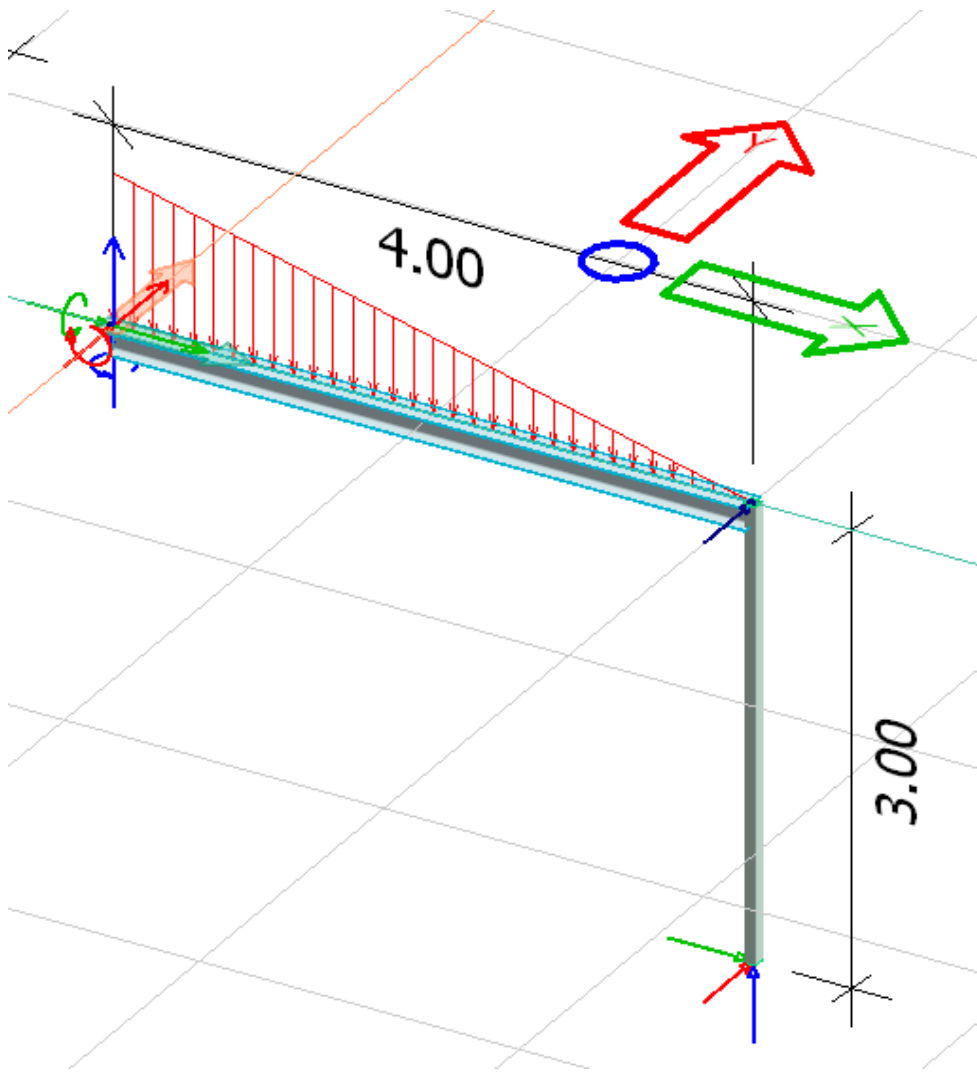


Løsningsforslag til opgave 11.2.1.

`with(Student[LinearAlgebra]) :`
`restart`

Bjælke er HEA160 og søjlen er en KKR 80x80x4. Der regnes med charnier i samlingen mellem elementerne, hvilket her klares ved at nedsætte inertimomentet for KKR til meget lille. Dette kan gøres, da der kun er normalkræfter i elementet. Efterfølgende ændres søjlen til HEA160 med momentstiv samling mellem bjælke og søjle.





Stivhedsmatrice for element 1

$$K_{11} := \begin{bmatrix} \frac{EA I}{L I} & 0 & 0 & -\frac{EA I}{L I} & 0 & 0 \\ 0 & \frac{12 E I I}{L I^3} & -\frac{6 E I I}{L I^2} & 0 & -\frac{12 E I I}{L I^3} & -\frac{6 E I I}{L I^2} \\ 0 & -\frac{6 E I I}{L I^2} & \frac{4 E I I}{L I} & 0 & \frac{6 E I I}{L I^2} & \frac{2 E I I}{L I} \\ -\frac{EA I}{L I} & 0 & 0 & \frac{EA I}{L I} & 0 & 0 \\ 0 & -\frac{12 E I I}{L I^3} & \frac{6 E I I}{L I^2} & 0 & \frac{12 E I I}{L I^3} & \frac{6 E I I}{L I^2} \\ 0 & -\frac{6 E I I}{L I^2} & \frac{2 E I I}{L I} & 0 & \frac{6 E I I}{L I^2} & \frac{4 E I I}{L I} \end{bmatrix} :$$

Puttematrice for K11

$$pI := \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} :$$

Stivhedsmtarice for element 2 i det lokale system

$$K12 := \begin{bmatrix} \frac{EA2}{L2} & 0 & 0 & -\frac{EA2}{L2} & 0 & 0 \\ 0 & \frac{12EI2}{L2^3} & -\frac{6EI2}{L2^2} & 0 & -\frac{12EI2}{L2^3} & -\frac{6EI2}{L2^2} \\ 0 & -\frac{6EI2}{L2^2} & \frac{4EI2}{L2} & 0 & \frac{6EI2}{L2^2} & \frac{2EI2}{L2} \\ -\frac{EA2}{L2} & 0 & 0 & \frac{EA2}{L2} & 0 & 0 \\ 0 & -\frac{12EI2}{L2^3} & \frac{6EI2}{L2^2} & 0 & \frac{12EI2}{L2^3} & \frac{6EI2}{L2^2} \\ 0 & -\frac{6EI2}{L2^2} & \frac{2EI2}{L2} & 0 & \frac{6EI2}{L2^2} & \frac{4EI2}{L2} \end{bmatrix} :$$

$$v := \frac{\pi}{2}$$

$$\frac{1}{2} \pi$$

(1)

Transformationsmatrice for element 1

$$T := \begin{bmatrix} \cos(v) & \sin(v) & 0 & 0 & 0 & 0 \\ -\sin(v) & \cos(v) & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & \cos(v) & \sin(v) & 0 \\ 0 & 0 & 0 & -\sin(v) & \cos(v) & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$\begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 \\ -1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

(2)

$$K22 := T^+ \cdot K12 \cdot T :$$

Puttematrice for K2

$$p2 := \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} :$$

$$K1 := p1 \cdot K11 \cdot p1^+ :$$

$$K2 := p2 \cdot K22 \cdot p2^+ :$$

Samlet stivhedsmatrice

$$K := K1 + K2 :$$

$$u := \begin{bmatrix} 0 \\ 0 \\ 0 \\ ubv \\ ubl \\ rb \\ 0 \\ 0 \\ r_c \end{bmatrix} :$$

$$Ur := \begin{bmatrix} Av \\ Al \\ MA \\ 0 \\ 0 \\ 0 \\ Cv \\ Cl \\ 0 \end{bmatrix} :$$

De virtuelle knudelaste fra den fordelte last.

$$p1 := \left(1 - \frac{x}{L1} \right) \cdot q :$$

$$U1 := \int_0^{L1} \left(1 - \frac{3 \cdot x^2}{L1^2} + \frac{2 \cdot x^3}{L1^3} \right) \cdot p1 \, dx ; U2 := \int_0^{L1} - \left(x - \frac{2 \cdot x^2}{L1} + \frac{x^3}{L1^2} \right) \cdot p1 \, dx ; U3 := \int_0^{L1} \left(\frac{3 \cdot x^2}{L1^2} - \frac{2 \cdot x^3}{L1^3} \right) \cdot p1 \, dx ;$$

$$U4 := \int_0^{Ll} \left(\frac{x^2}{Ll} - \frac{x^3}{Ll^2} \right) \cdot p l \, dx :$$

$U1, U2, U3, U4$

$$\frac{7}{20} Ll q, -\frac{1}{20} Ll^2 q, \frac{3}{20} Ll q, \frac{1}{30} Ll^2 q \quad (3)$$

$$U_v := \begin{bmatrix} 0 \\ U1 \\ U2 \\ 0 \\ U3 \\ U4 \\ 0 \\ 0 \\ 0 \end{bmatrix}$$

$$\begin{bmatrix} 0 \\ \frac{7}{20} Ll q \\ -\frac{1}{20} Ll^2 q \\ 0 \\ \frac{3}{20} Ll q \\ \frac{1}{30} Ll^2 q \\ 0 \\ 0 \\ 0 \end{bmatrix} \quad (4)$$

$U := U_r + U_v$

$$\begin{bmatrix} A_v \\ A_l + \frac{7}{20} Ll q \\ M A - \frac{1}{20} Ll^2 q \\ 0 \\ \frac{3}{20} Ll q \\ \frac{1}{30} Ll^2 q \\ C_v \\ C_l \\ 0 \end{bmatrix} \quad (5)$$

Løsning af det modificerede system.

Puttematrice pmod

$$pmod := \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 \end{bmatrix} :$$

$$Kmod := pmod^+ \cdot K \cdot pmod$$

$$\begin{bmatrix} \frac{EA}{L} + \frac{12EI}{L^3} & 0 & \frac{6EI}{L^2} & \frac{6EI}{L^2} \\ 0 & \frac{12EI}{L^3} + \frac{EA}{L} & \frac{6EI}{L^2} & 0 \\ \frac{6EI}{L^2} & \frac{6EI}{L^2} & \frac{4EI}{L} + \frac{4EI}{L} & \frac{2EI}{L} \\ \frac{6EI}{L^2} & 0 & \frac{2EI}{L} & \frac{4EI}{L} \end{bmatrix}$$

(6)

$$Umod := \begin{bmatrix} U_4 \\ U_5 \\ U_6 \\ U_9 \end{bmatrix}; umod := Kmod^{-1} \cdot Umod :$$

$$\begin{bmatrix} 0 \\ \frac{3}{20} Lq \\ \frac{1}{30} L^2 q \\ 0 \end{bmatrix}$$

(7)

$$u := \begin{bmatrix} 0 \\ 0 \\ 0 \\ umod_1 \\ umod_2 \\ umod_3 \\ 0 \\ 0 \\ umod_4 \end{bmatrix} :$$

$$U := K_u :$$

$$UR := U - U_v :$$

Taleksempel 1.

Element 1 er HEA160 og element 2 er KKR 80x80x4.

$$I1 := 16700000; E := 210000; L1 := 4000; A1 := 3877; L2 := 3000; q := 40; I2 := \frac{I1}{10000000}; A2 := 1175$$

$$16700000$$

$$210000$$

$$4000$$

$$3877$$

$$3000$$

$$40$$

$$\frac{167}{100}$$

$$1175$$

$$1175$$

(8)

$$\frac{E \cdot A2}{L2}$$

$$82250$$

(9)

$$UR := U - U_v :$$

$$evalf_3(UR \cdot 1.0)$$

$$\begin{bmatrix} 0.000703 \\ -64000. \\ 4.28 \cdot 10^7 \\ 0. \\ 0. \\ 0. \\ -0.000703 \\ -16000. \\ 0. \end{bmatrix}$$

(10)

$$(UR_2 + UR_8) \cdot 1.0$$

$$-80000.00000$$

(11)

Fra FEM-Design

Point support group, reactions, Load comb.: Lodret

ID	x	y	z	Node	Fx'	Fy'	Fz'	Mx'	My'	Mz'
[-]	[m]	[m]	[m]	[-]	[kN]	[kN]	[kN]	[kNm]	[kNm]	[kNm]
S.1	0.000	0.000	0.000	1	0.000	0.000	-64.027	0.000	42.775	0.000
S.3	4.000	0.000	-3.000	3	0.000	0.000	-15.973	0.000	0.000	0.000

Taleksempel.

Element 1 er HEA160 og element 2 er HEA160 med momentstiv samling.

$l1 := 16700000; E := 210000; L1 := 4000; A1 := 3877; L2 := 3000; q := 40; l2 := l1; A2 := A1$

$$\begin{aligned} &16700000 \\ &210000 \\ &4000 \\ &3877 \\ &3000 \\ &40 \\ &16700000 \\ &3877 \end{aligned} \tag{12}$$

$$\frac{E \cdot A2}{L2} = 271390 \tag{13}$$

$UR := U - Uv :$

$evalf_3(UR \cdot 1.0)$

$$\begin{bmatrix} 3540. \\ -60000. \\ 3.74 \cdot 10^7 \\ 0. \\ 0. \\ 0. \\ -3540. \\ -20000. \\ 0. \end{bmatrix} \tag{14}$$

$$(UR_2 + UR_8) = -80000 \tag{15}$$

Fra FEM-Design

Point support group, reactions, Load comb.: Lodret

ID	x	y	z	Node	Fx'	Fy'	Fz'	Mx'	My'	Mz'
[-]	[m]	[m]	[m]	[-]	[kN]	[kN]	[kN]	[kNm]	[kNm]	[kNm]
S.1	0.000	0.000	0.000	1	-3.538	0.000	-60.029	0.000	37.396	0.000
S.3	4.000	0.000	-3.000	3	3.538	0.000	-19.971	0.000	0.000	0.000

Kontrol af stivhedsmatrice

K =

Columns 1	through 7									
2,04E+08	0	0	-2,04E+08	0	0	0	0	0	0	0
0	657.6	1,32E+09	0	-657.6	1,32E+09	0	0	0	0	0
0	1,32E+09	3,51E+12	0	-1,32E+09	1,75E+12	0	0	0	0	0
-2,04E+08	0	0	2,05E+08	0	2,34E+09	-1559	0	2,34E+09	0	2,34E+09
0	-657.6	-1,32E+09	0	2.72e+05	-1,32E+09	0	-2,71E+08	0	0	0
0	1,32E+09	1,75E+12	2,34E+09	-1,32E+09	8,18E+12	-2,34E+09	0	2,34E+12	0	2,34E+12
0	0	0	-1559	0	-2,34E+09	1559	0	-2,34E+09	0	-2,34E+09
0	0	0	0	-2,71E+08	0	0	2,71E+08	0	0	0
0	0	0	2,34E+09	0	2,34E+12	-2,34E+09	0	4,68E+12	0	4,68E+12

$evalf_3(K \cdot 1000.0)$

$$\begin{bmatrix} 2.04 \cdot 10^8 & 0. & 0. & -2.04 \cdot 10^8 & 0. & 0. & 0. & 0. & 0. \\ 0. & 6.58 \cdot 10^5 & -1.32 \cdot 10^9 & 0. & -6.58 \cdot 10^5 & -1.32 \cdot 10^9 & 0. & 0. & 0. \\ 0. & -1.32 \cdot 10^9 & 3.51 \cdot 10^{12} & 0. & 1.32 \cdot 10^9 & 1.75 \cdot 10^{12} & 0. & 0. & 0. \\ -2.04 \cdot 10^8 & 0. & 0. & 2.05 \cdot 10^8 & 0. & 2.34 \cdot 10^9 & -1.56 \cdot 10^6 & 0. & 2.34 \cdot 10^9 \\ 0. & -6.58 \cdot 10^5 & 1.32 \cdot 10^9 & 0. & 2.72 \cdot 10^8 & 1.32 \cdot 10^9 & 0. & -2.71 \cdot 10^8 & 0. \\ 0. & -1.32 \cdot 10^9 & 1.75 \cdot 10^{12} & 2.34 \cdot 10^9 & 1.32 \cdot 10^9 & 8.18 \cdot 10^{12} & -2.34 \cdot 10^9 & 0. & 2.34 \cdot 10^{12} \\ 0. & 0. & 0. & -1.56 \cdot 10^6 & 0. & -2.34 \cdot 10^9 & 1.56 \cdot 10^6 & 0. & -2.34 \cdot 10^9 \\ 0. & 0. & 0. & 0. & -2.71 \cdot 10^8 & 0. & 0. & 2.71 \cdot 10^8 & 0. \\ 0. & 0. & 0. & 2.34 \cdot 10^9 & 0. & 2.34 \cdot 10^{12} & -2.34 \cdot 10^9 & 0. & 4.68 \cdot 10^{12} \end{bmatrix}$$

(16)